

FLOOD PLAIN INFORMATION STUDY

SALINAS NATIONAL MONUMENT

Part 1 of 2

ABO UNIT

Vicinity of Scholle, New Mexico

Ref. MT 7072-2-0060 NPS

Prepared for

U.S. Department of the Interior

National Park Service

by

Department of the Army

Albuquerque District Corps of Engineers

Albuquerque, New Mexico



Introduction

This flood study was prepared by the Albuquerque District U. S. Army Corps of Engineers (COE) for the U. S. Department of the Interior National Park Service (NPS) Southwest Regional Office. The study was authorized in a letter by Mr. Jack Nechels of the NPS dated August 12, 1982. The hydrologic and hydraulic computations were performed by Thomas Ryan, Hydraulic Engineer, of the Hydrologic Engineering Section, Planning Branch, Engineering and Planning Division, COE.

The study addresses the requirements set forth by Executive Order 11988. Briefly, Section 2(a)(1) of E.O. 11988 (May 24, 1977) requires that, "Before taking an action (" . . . Federally undertaken, financed or assisted construction and improvements," Section 1), each [federal] agency shall determine whether the proposed action will occur in a floodplain . . ." The term floodplain is defined "as a minimum, areas subject to inundation by a flood with a one percent chance of occurring in any year (i.e., "100-year or base flood")."

The Salinas National Monument, located in Torrance and Socorro Counties of New Mexico, contains three separate units Abo, Quarai and Gran Quivira. This report is Part 1 of a two-part report and covers only the Abo Unit in Torrance County near Scholle, New Mexico. Part 2 of this report will be on the Quarai Unit in Torrance County near Punta de Agua, New Mexico. The Gran Quivira Unit in Socorro County near Gran Quivira, New Mexico, was not requested under this contract and will not be presented.

Mr. Nechels' previously mentioned letter requested the "2-, 5-, 10-, 50-, 100-, 500- year frequency and Q maximum flood events for both the Abo and Quarai Units" (Salinas National Monument). No need was seen to evaluate the Gran Quivira Unit at present. 'Q maximum' was interpreted to mean probable maximum flood (PMF) peak. PMF is defined as the runoff from the theoretically greatest depth of precipitation for a given duration that is physically possible over a given size storm area at particular geographical location at a certain time of the year. It was agreed upon verbally between Mr. Garland (Gary) Moore, NPS, and Mr. Thomas Ryan, COE, that only the peak discharge for each event would be calculated and presented.

Mapping for this study and report was provided by the NPS to the COE. The work maps are at a scale of 1 in. = 50 ft. with a contour interval of 1 ft. and are based on aerial photography exposed on September 29, 1978. The mapping was produced under a NPS contract to the Remote Sensing Division of the NPS located at the Bandelier West Building on the University of New Mexico Campus, Albuquerque, New Mexico. The datum is arbitrary and is in no way related to mean sea level. No attempt was made by the COE or the NPS or any other agency to reference the arbitrary datum to a national or state datum. The initial contracting officer for the NPS was Mr. Garland Moore.



Abo Unit, Salinas National Monument

1. Hydrology:

There are no stream gages on any of the flowpaths in the study area. Runoff patterns in the area by observation are due to thunderstorms of varying intensities with the streambeds remaining dry a large portion of the time. No significant detention or diversion structures were found in any portion of the watershed.

Figures 1 through 3 following this text are respectively Regional, Location and Site maps of the project area. Figure 4 is an area map with the major concentration points where peak flows were calculated. Figure 4 also shows the stream numbering system used in this report. The main arroyo, Cañon Espinoso, is labeled Stream 1000 with its tributaries being designated 2000, 3000 and 5000. Stream 4000 is a tributary to Stream 3000.

Peak discharges for the 2-, 5-, 10-, 50-, 100-, 500 year floods were computed using equations and procedures contained in the U. S. Geological Survey's manual, Techniques for Estimating Flood Discharges for Unregulated Streams in New Mexico, Water Resources Investigations 82-24, published in March 1982. The manual contains equations for estimating flood magnitude at selected recurrence intervals from 2 to 500 years. For all drainage areas greater than one square mile, the equations used for this study are found on page 9 of the manual. The two variables, drainage area and site altitude, were taken at various concentration points from U.S. Geological Survey (USGS) 7-1/2 minute quadrangle sheets of the area (see figure 4). After the peak flows were calculated, expected probability was added according to the procedure found in Appendix 11 of Guidelines for Determining Flood Flow Frequency, Bulletin #17B, Revised September 1981 with Editorial Corrections March 1982, by Interagency Advisory Committee on Water Data published by U. S. Department of the Interior, USGS, Office of Water Data Coordination. The frequency peak flows for drainage areas less than one square mile were computed by first plotting all peak flows, including representative PMF peaks, on logarithmic paper, plotting drainage area versus discharge per square mile. The PMF peaks were obtained from previous Corps' studies and represent drainage areas between 0.13 and 26 square miles. A best fit curve was drawn. The frequency curves for the drainage areas below one square mile were adjusted to reflect the same curvature as the PMF curve below one square mile. The final series of curves are shown in figure 5. The corresponding peak flows for the 2-, 5-, 10-, 50-, 100-, 500- year floods and PMF are shown in Table 1 as follows:



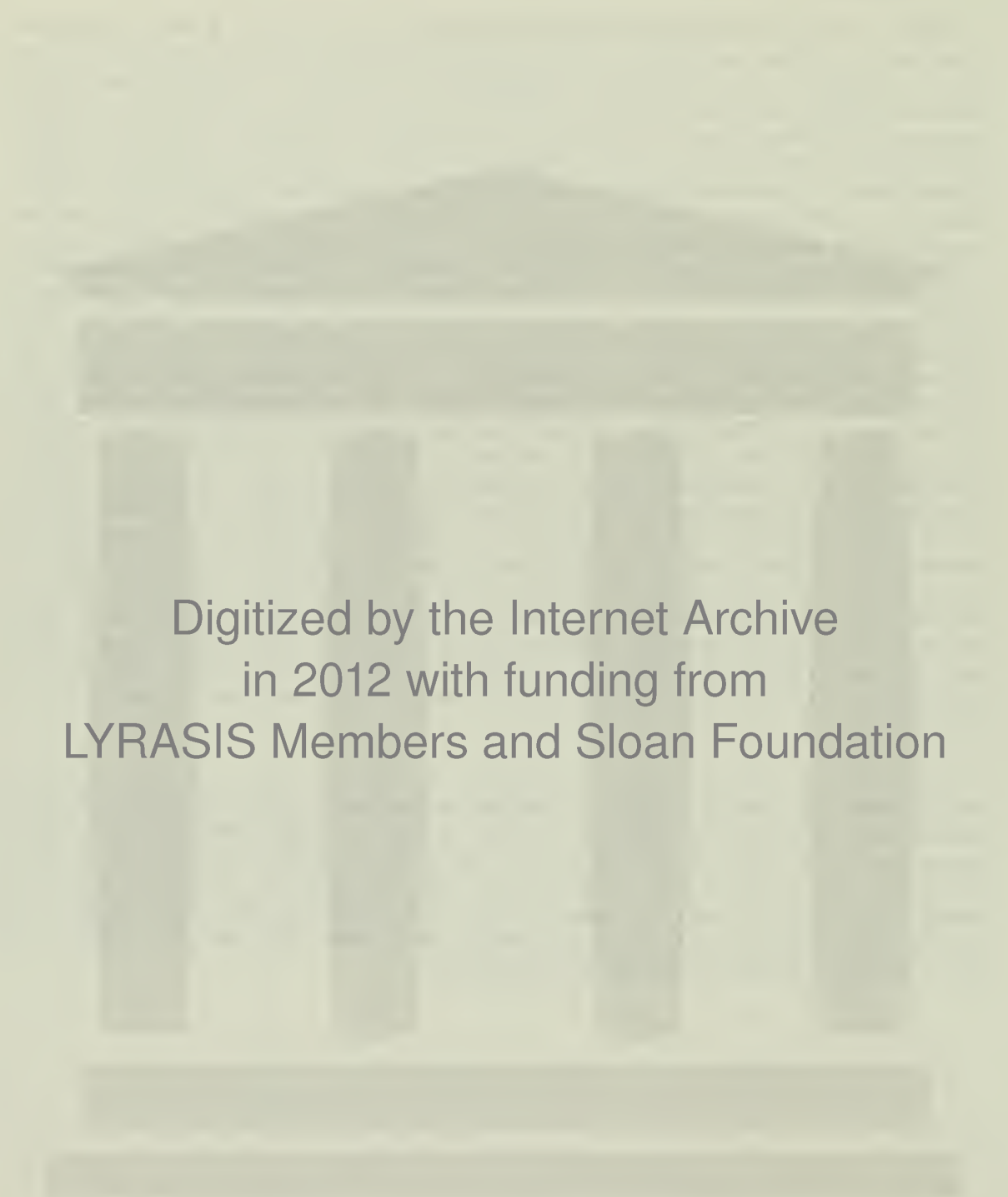
Table 1
Peak Flows in C.F.S.

Concentration Point Number	Stream	Peak Flows (CFS)						
		Q2	Q5	Q10	Q50	Q100	Q500	QPMF
1	1000	275	680	1130	2800	3950	8100	62100
2	1000	269	670	1115	2710	3850	7800	61600
2a	1000	268	665	1110	2700	3750	7700	61350
2b	2000	14	42	73	201	304	690	2300
3	1000	267	660	1100	2700	3700	7500	61300
3a	1000	235	600	990	2500	3500	7100	53350
3b	3000	122	330	560	1500	2100	4450	24500
4	1000	231	595	970	2400	3330	6750	53100
4a	5000	14	40	70	193	290	662	2200
4b	1000	229	590	955	2310	3300	6700	53000
5	1000	229	570	950	2300	3280	6690	52950
6	5000	12	37	64	174	264	600	2000
7	3000	120	325	555	1450	2100	4400	24400
7a	3000	118	320	540	1430	2050	4300	23900
7b	4000	12	37	64	174	264	600	2000
8	2000	5	14	25	68	103	217	750
9	4000	10	31	54	147	224	507	1700
10	3000	115	310	515	1370	1970	4200	23800

2. Hydraulics

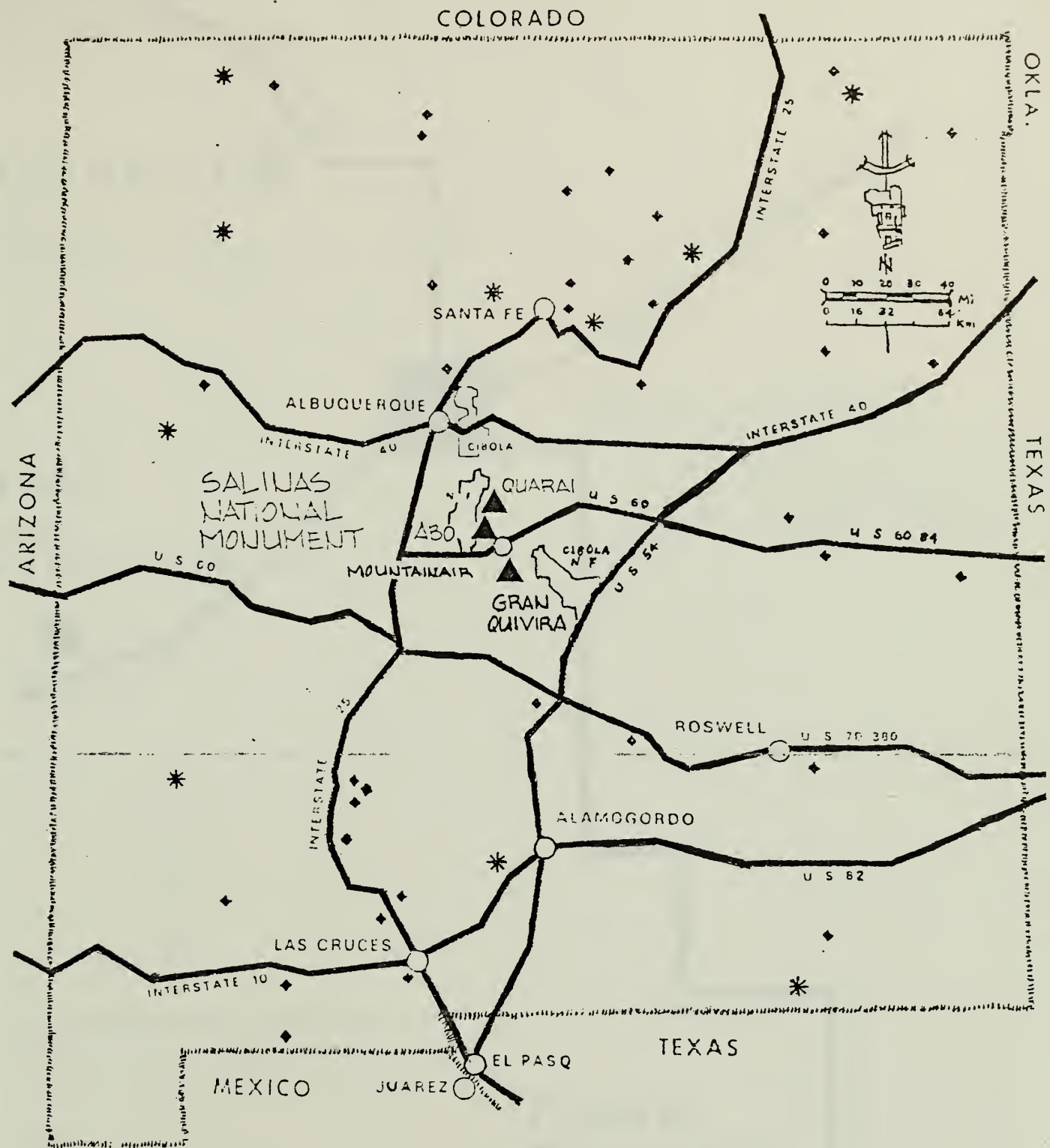
Water surface elevations were calculated using the COE's HEC-2 computer program. Cross sections were taken from the work maps provided by the NPS. Channel and overbank roughness values were based on field observations and expressed as Manning's 'n' values. Calculations were made in a backwater mode. Where the streambed exceeded critical slope the minimum calculated water surface elevation used was critical depth. Experience, observations and engineering judgment have shown that a natural channel can rarely, if ever, sustain supercritical flow over an appreciable distance and/or time period in the non-turbulent flow range. Critical depth is used to provide a safety factor for the obviously turbulent, unstable flow. In reaches where the streambed has a less than critical slope the water surface depth will be greater than critical depth. Flood depths may be determined at a particular point from the flood profiles on plates 1 through 14. The streambed profile shown on those plates are based on the lowest point in the streambed along each cross section. The actual ground may vary from the profile for a number of reasons. These include, but are not limited to, errors in mapping, changes in the streambed since mapping was accomplished, "straightlining" the profile between cross sections and the estimations of lowest point elevation taken from the mapping. However, these errors should not appreciably affect the flood depths at any particular point along the streams. The arroyo beds are being eroded at varying rates. Occasionally, the channel walls fracture with some large rock mass falling into the flow path. These channel features make the flood depths and flood plains susceptible to sudden change. However, except in extremely rare circumstances, the changes are not anticipated to be significant.

Figure 6 is the 100-year frequency floodplain map of the Abo Unit which shows the study limits. The PMF peaks were calculated for all flow paths but the PMF backwater run for Cañon Espinosa resulted in a basin overflow at the upper portion of the study. The NPS representative agreed to forego the requested PMF backwater calculation for simplification and cost savings in trying to figure the flow split and subsequent reduction in peak discharge on stream 1000 and the impact, if any, on the PMF peak flow on stream 3000.



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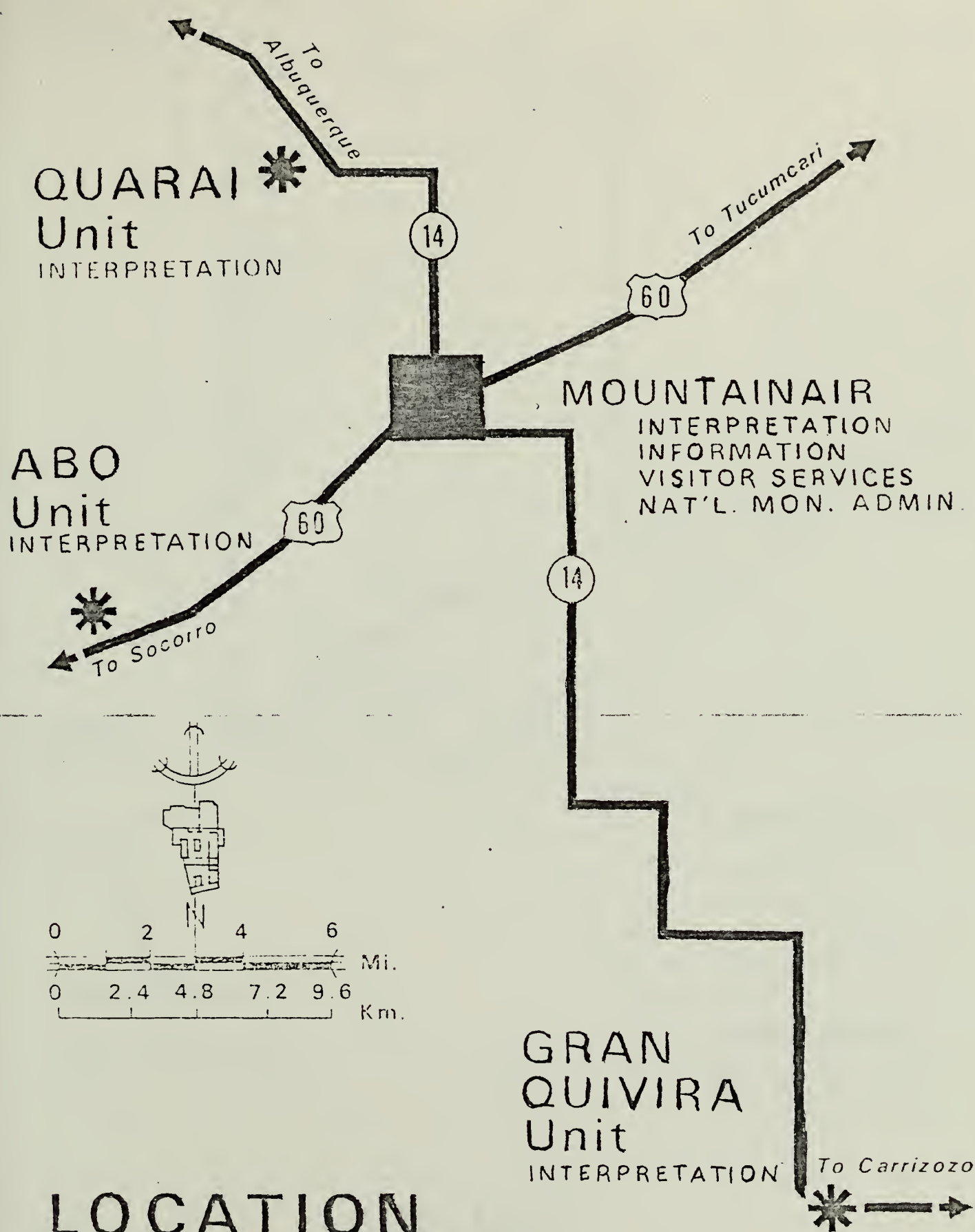
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THE REGION

Salt Lake National Monument, New Mexico
United States Department of the Interior / NPS

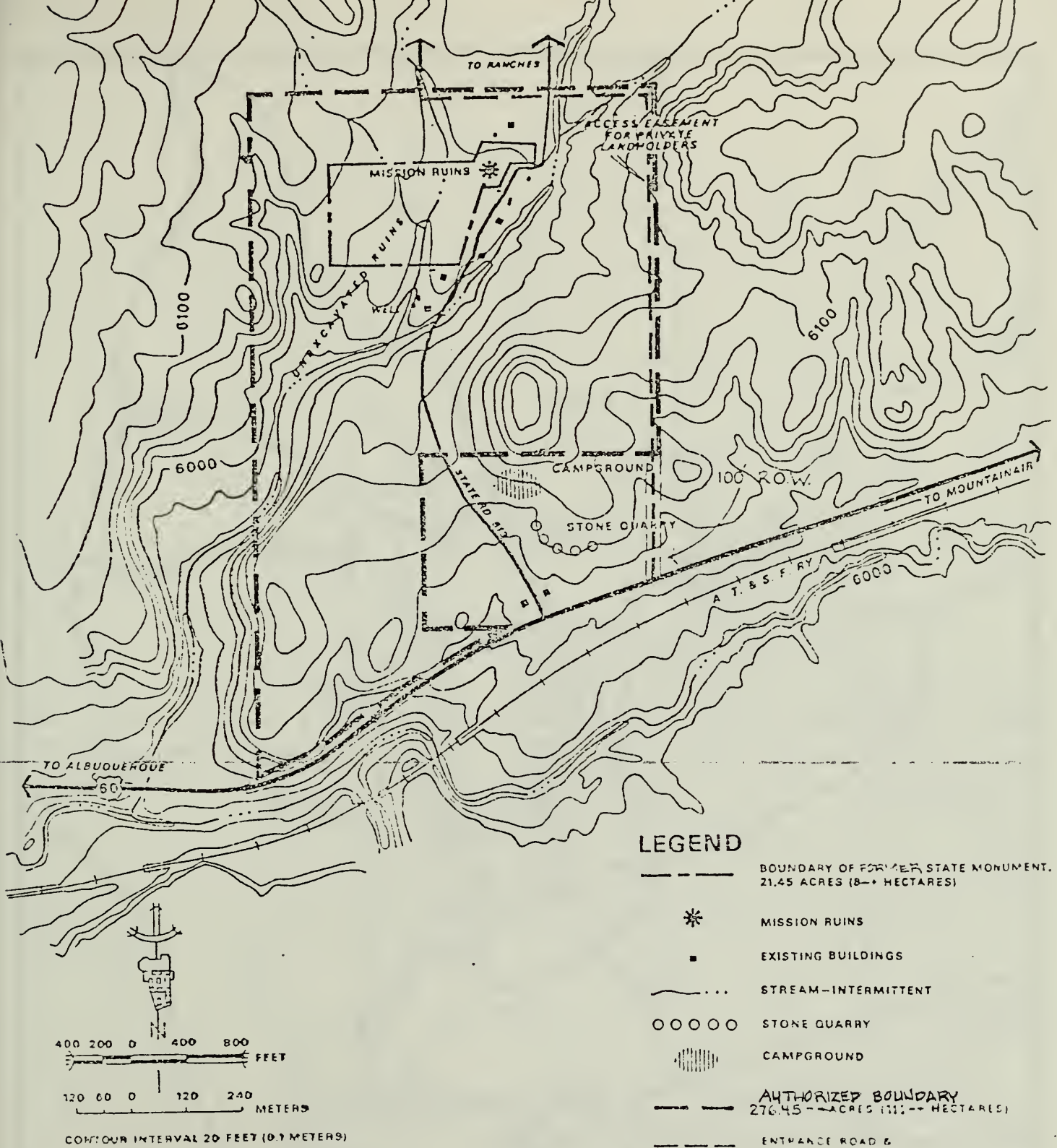
FIGURE 1



LOCATION

Salinas National Monument, New Mexico
United States Department of the Interior/NPS

FIGURE 2



ABO UNIT

Salinas National Monument, New Mexico
United States Department of the Interior/NPS





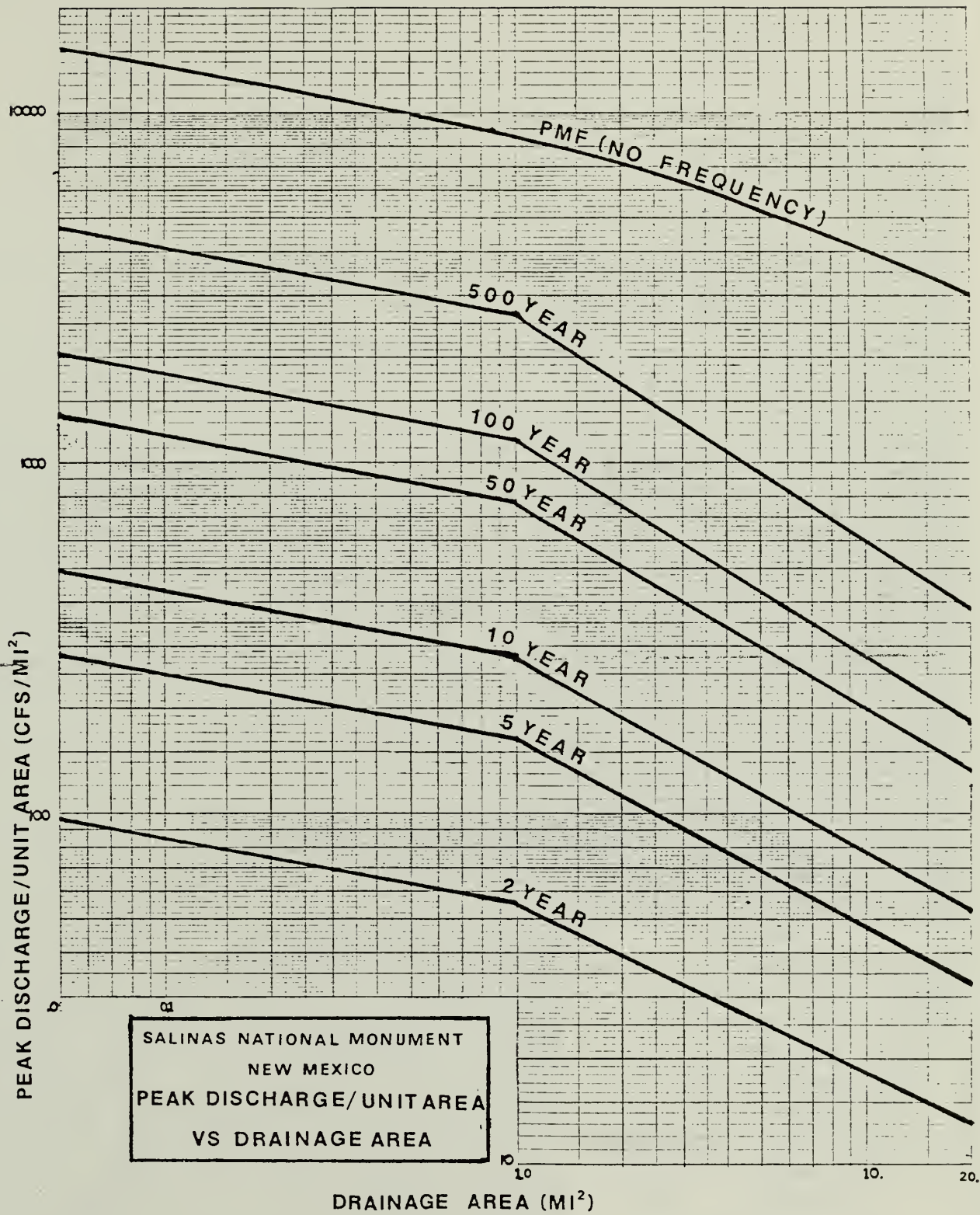
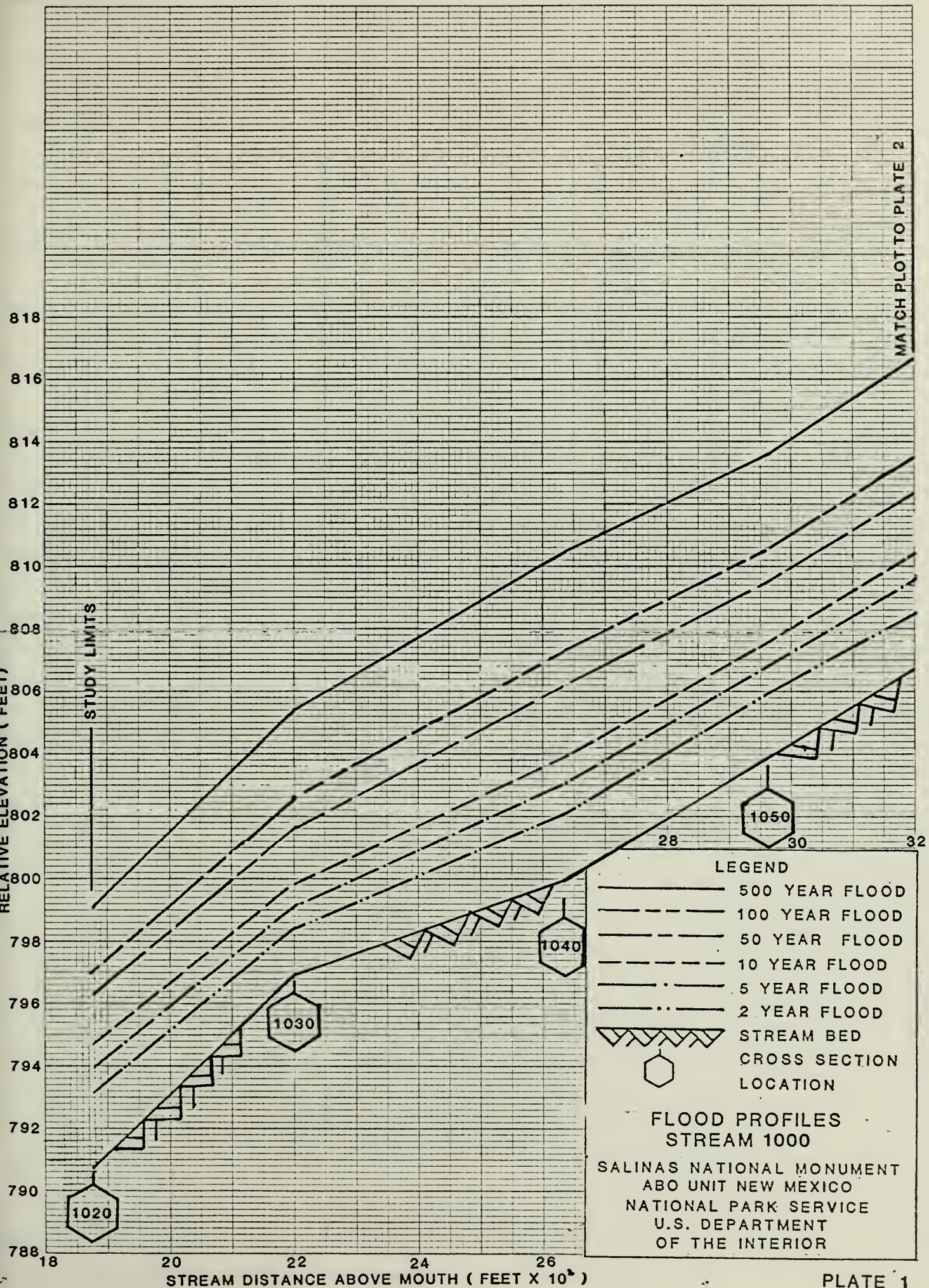
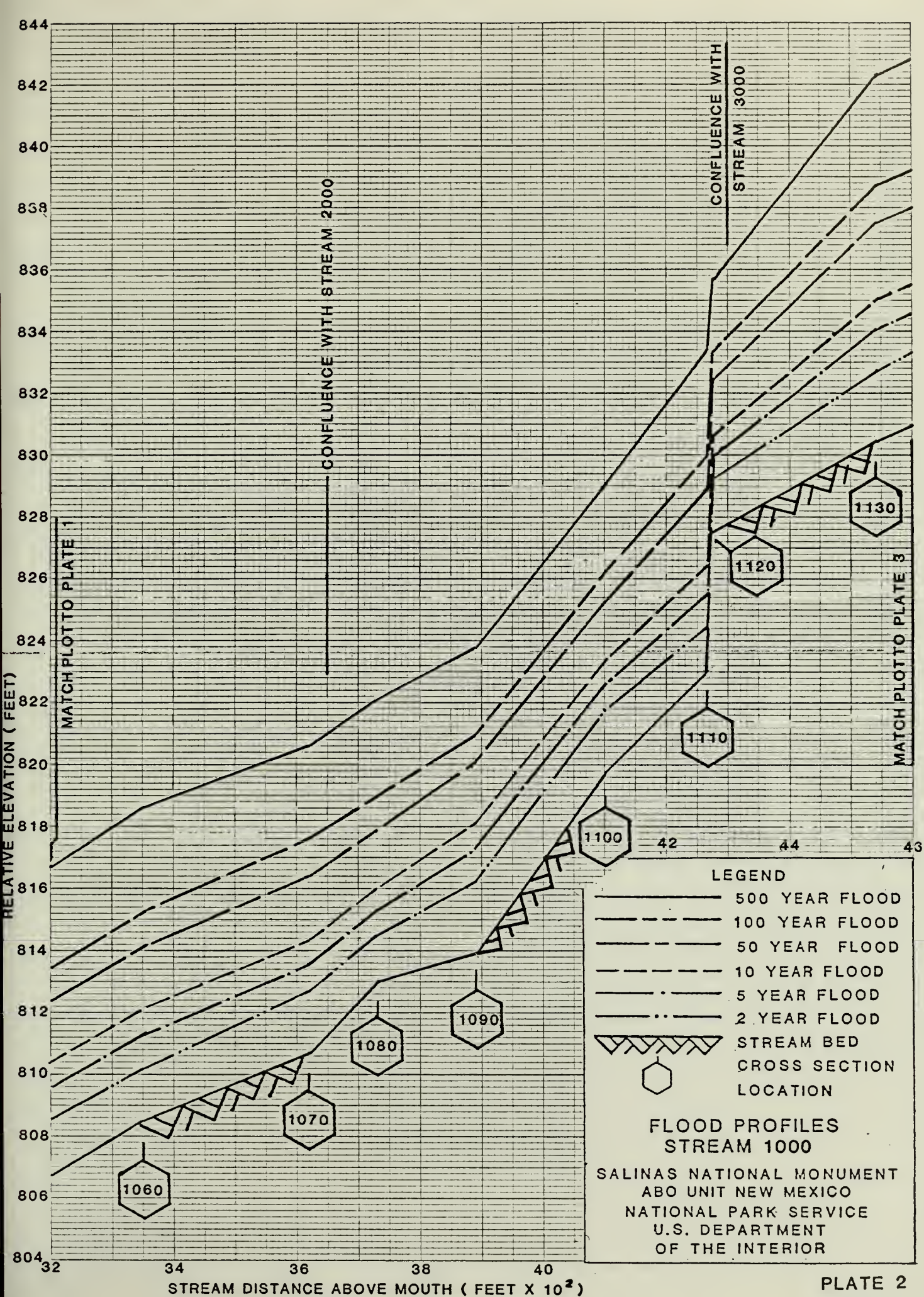
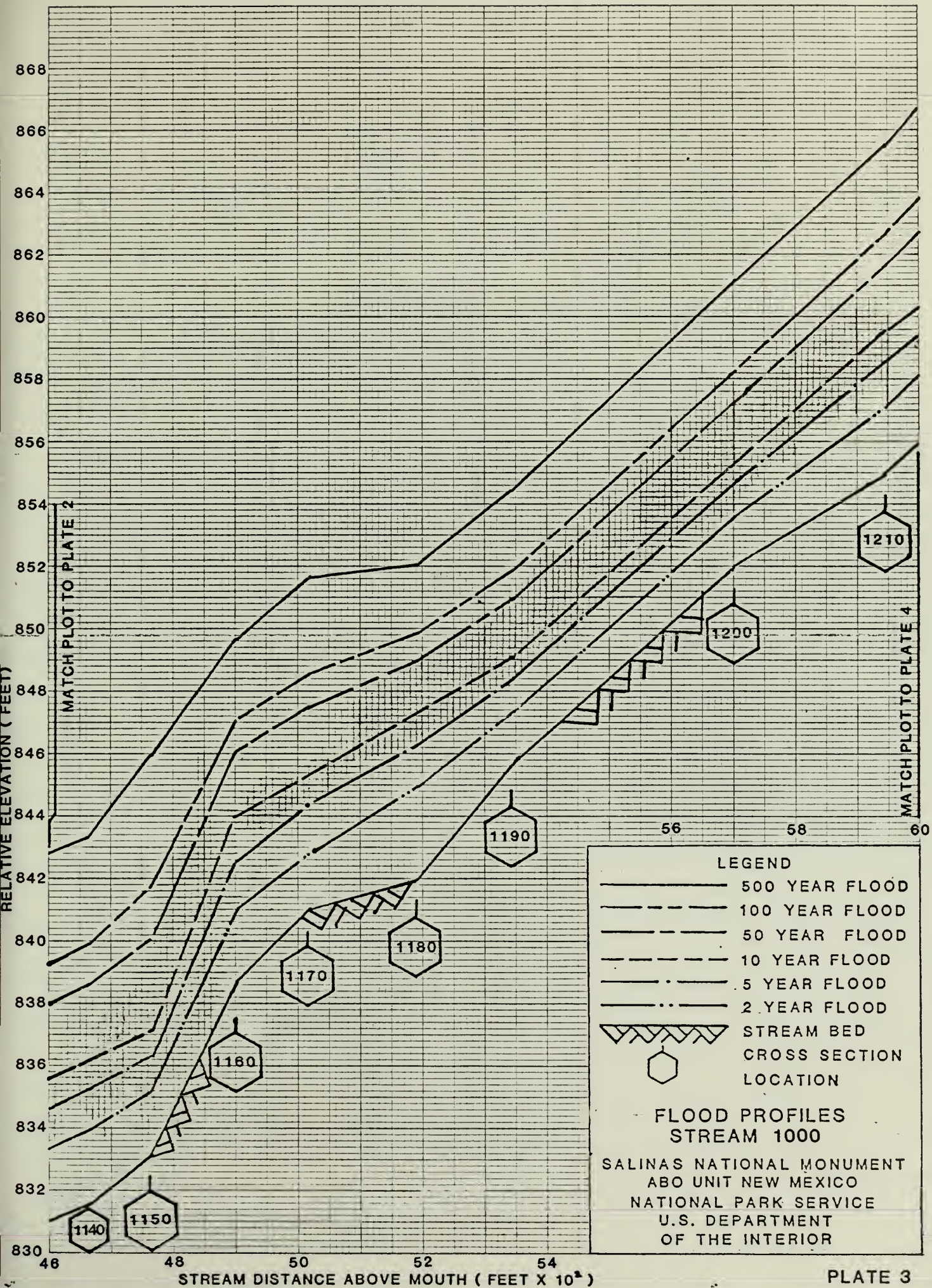


FIGURE 5





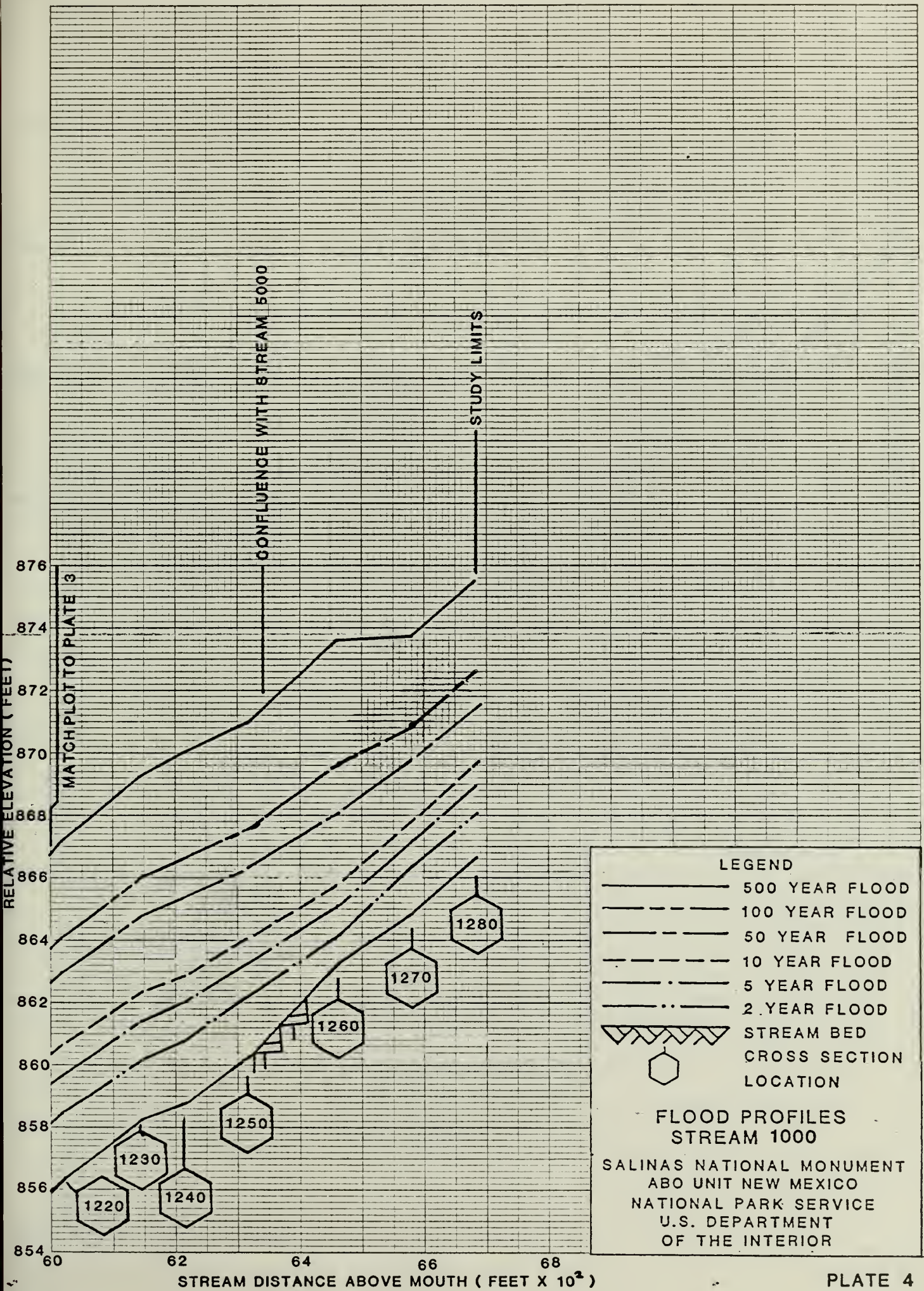


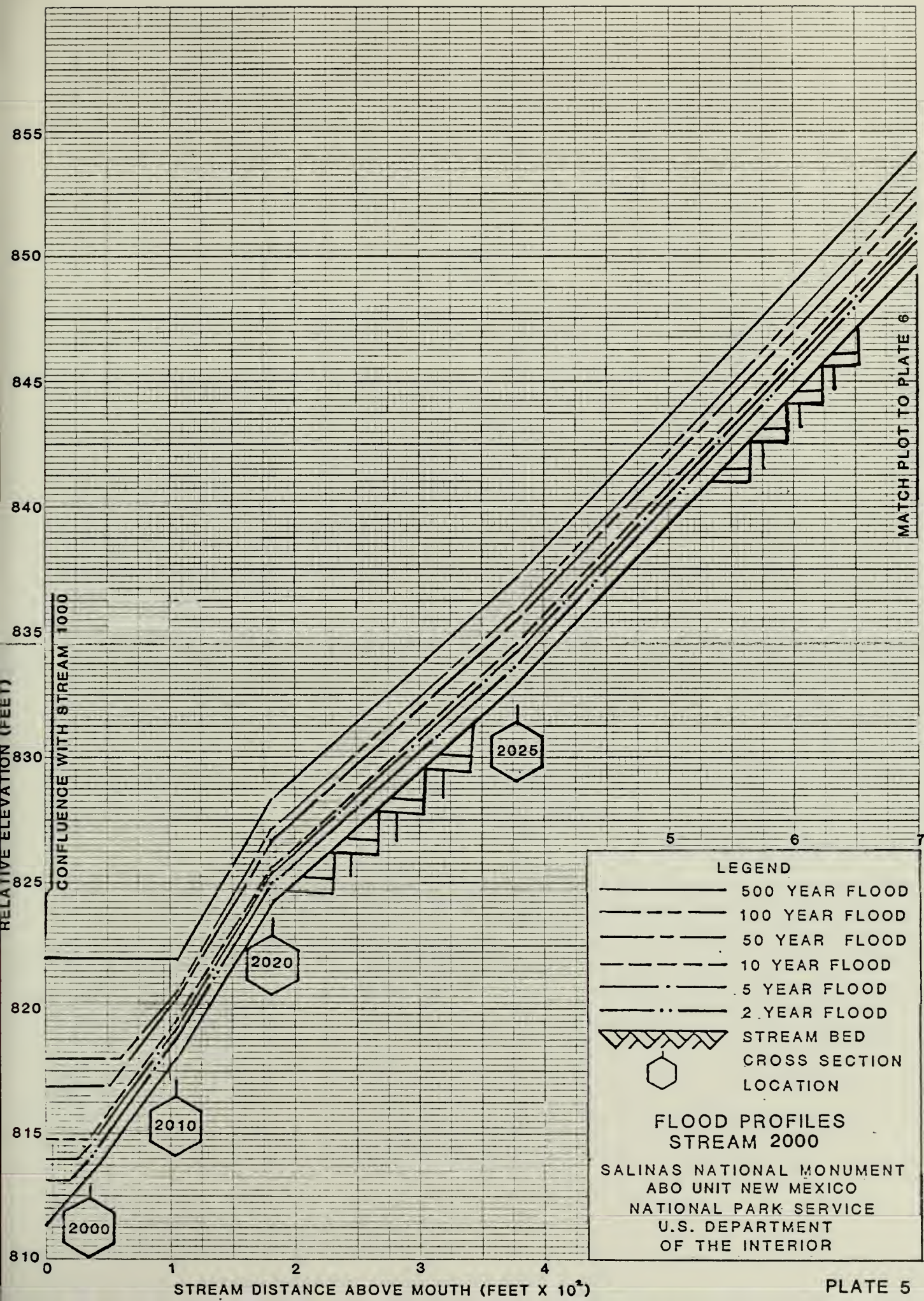
LEGEND

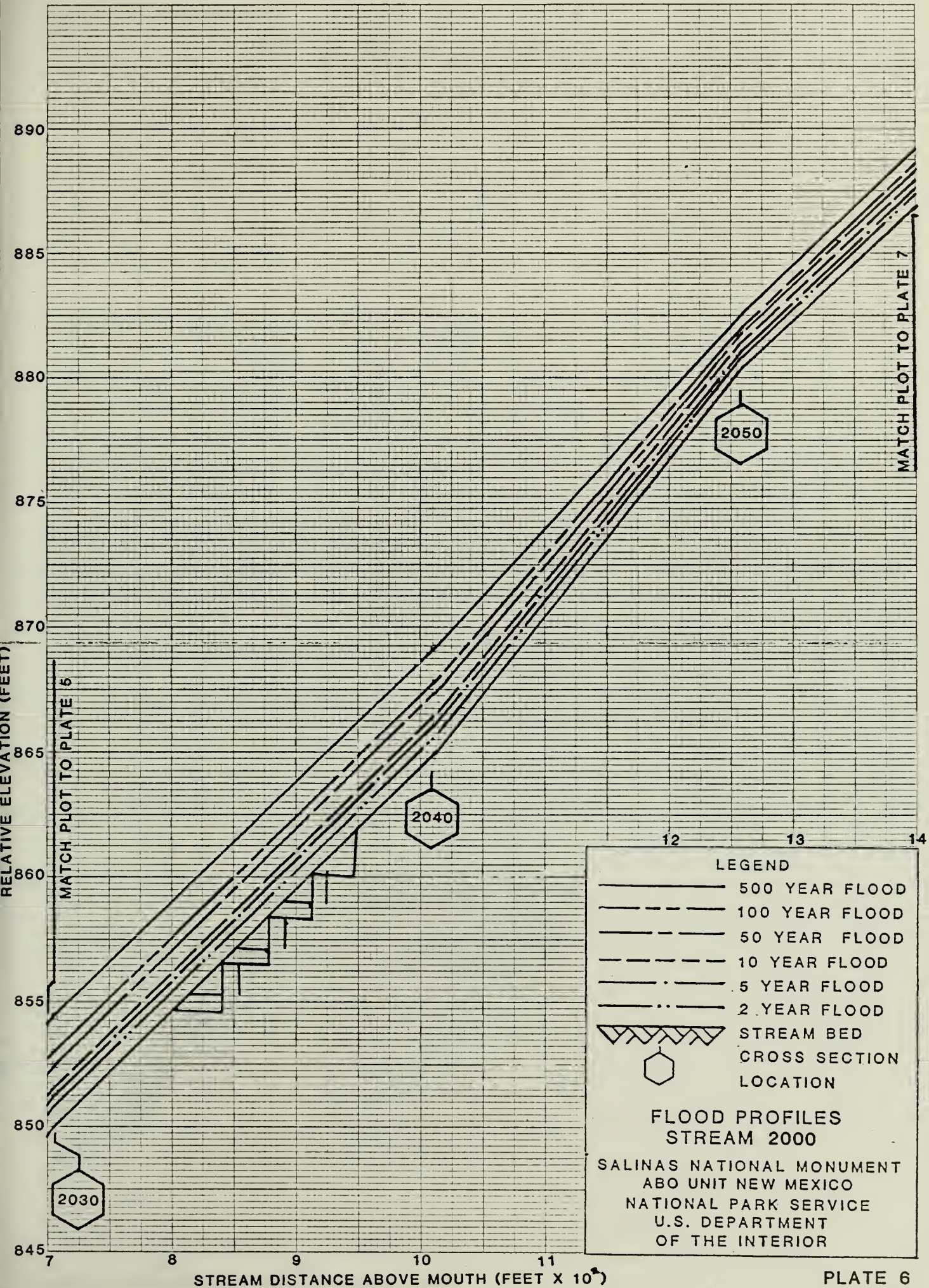
- 500 YEAR FLOOD
- 100 YEAR FLOOD
- 50 YEAR FLOOD
- 10 YEAR FLOOD
- 5 YEAR FLOOD
- 2 YEAR FLOOD
- STREAM BED
- CROSS SECTION LOCATION

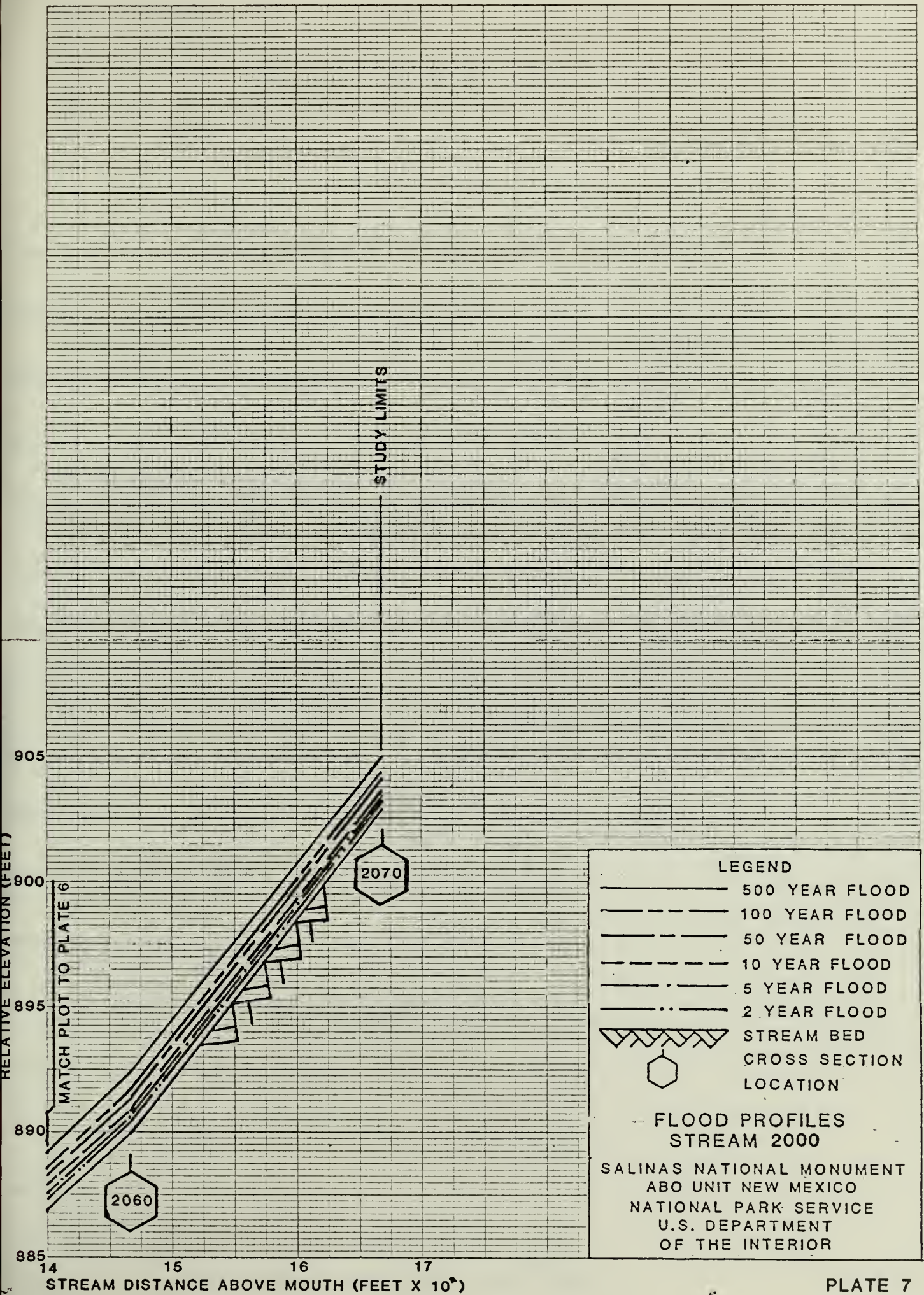
**FLOOD PROFILES
STREAM 1000**

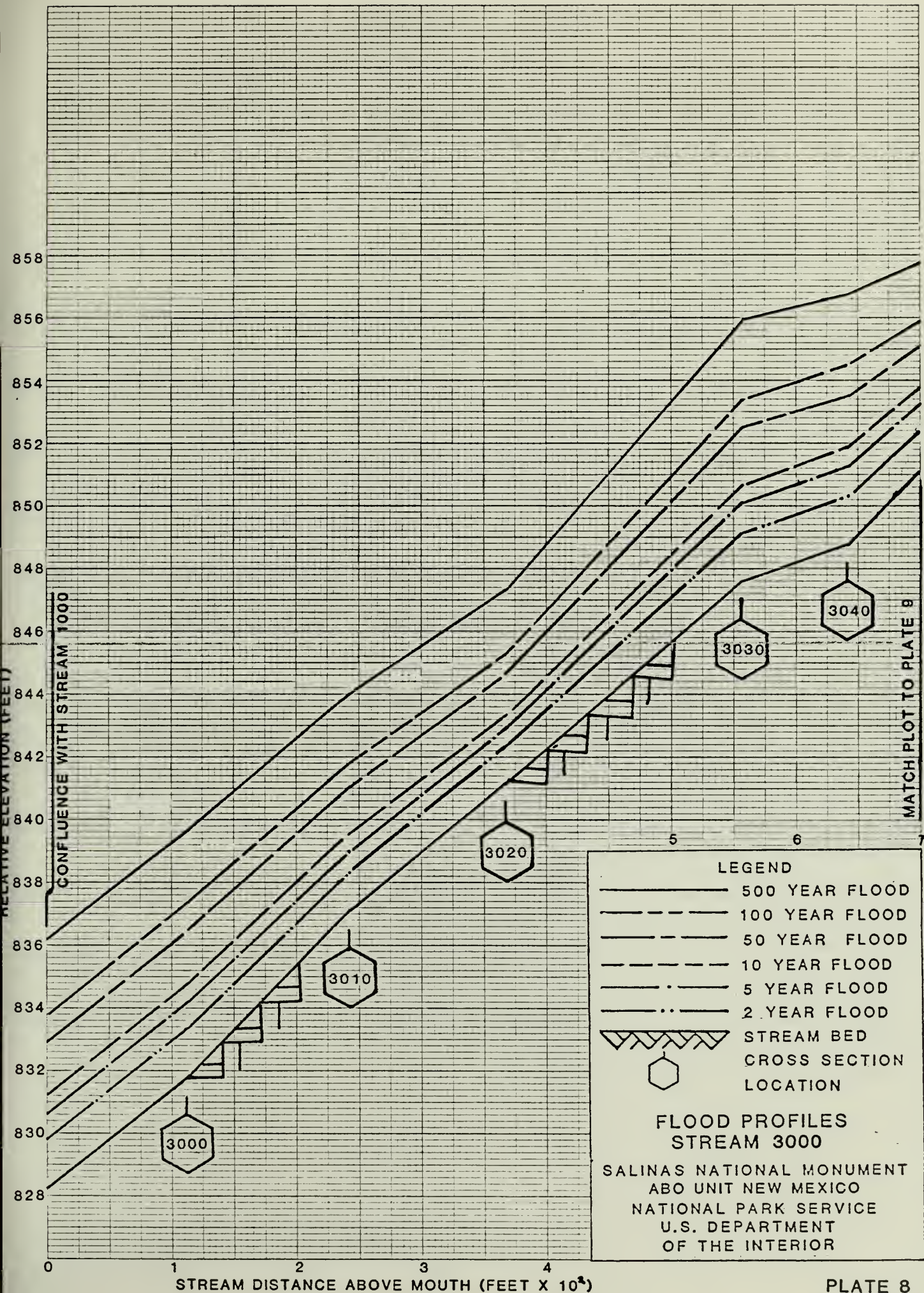
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NATIONAL PARK SERVICE
U.S. DEPARTMENT
OF THE INTERIOR

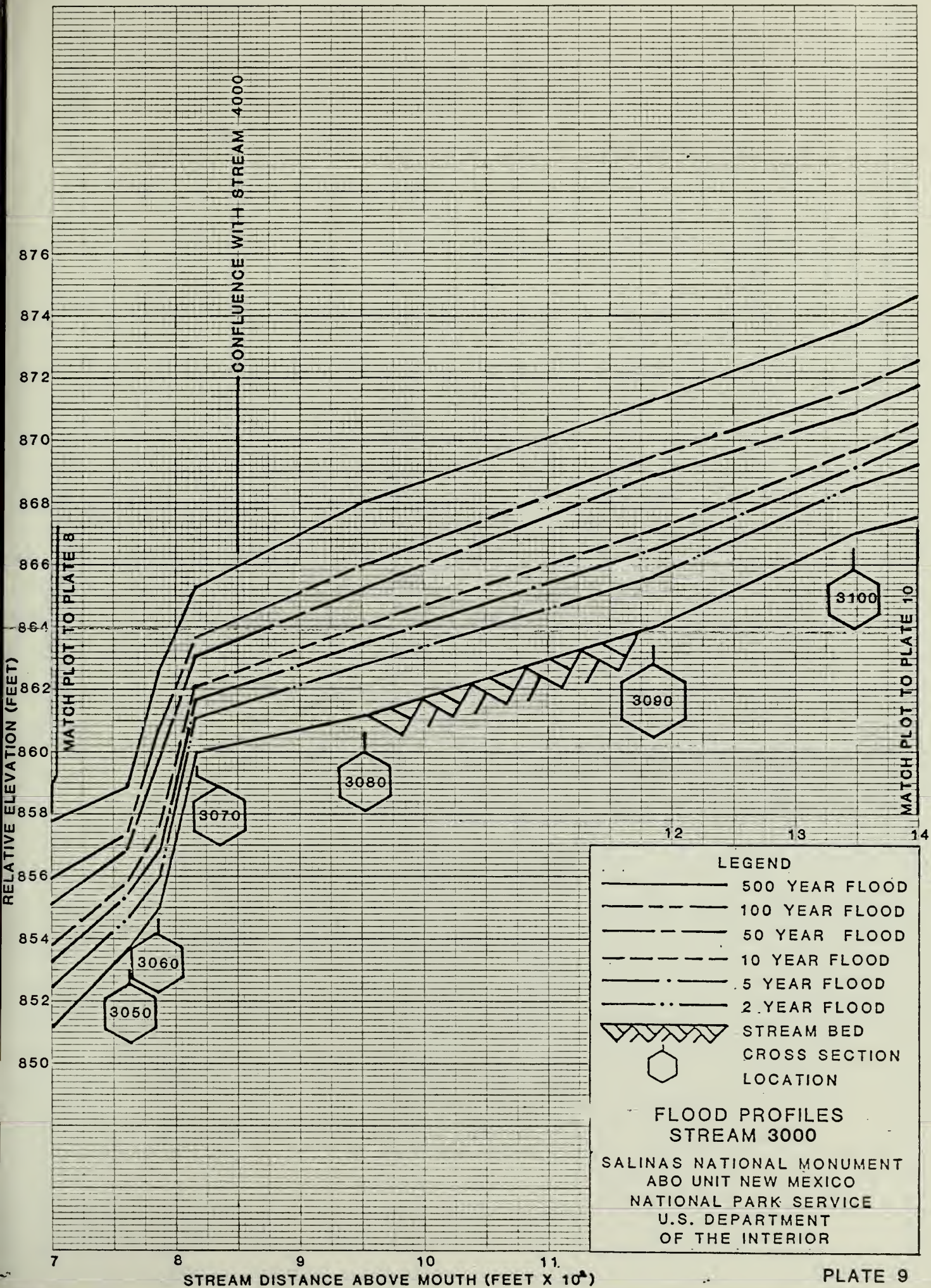


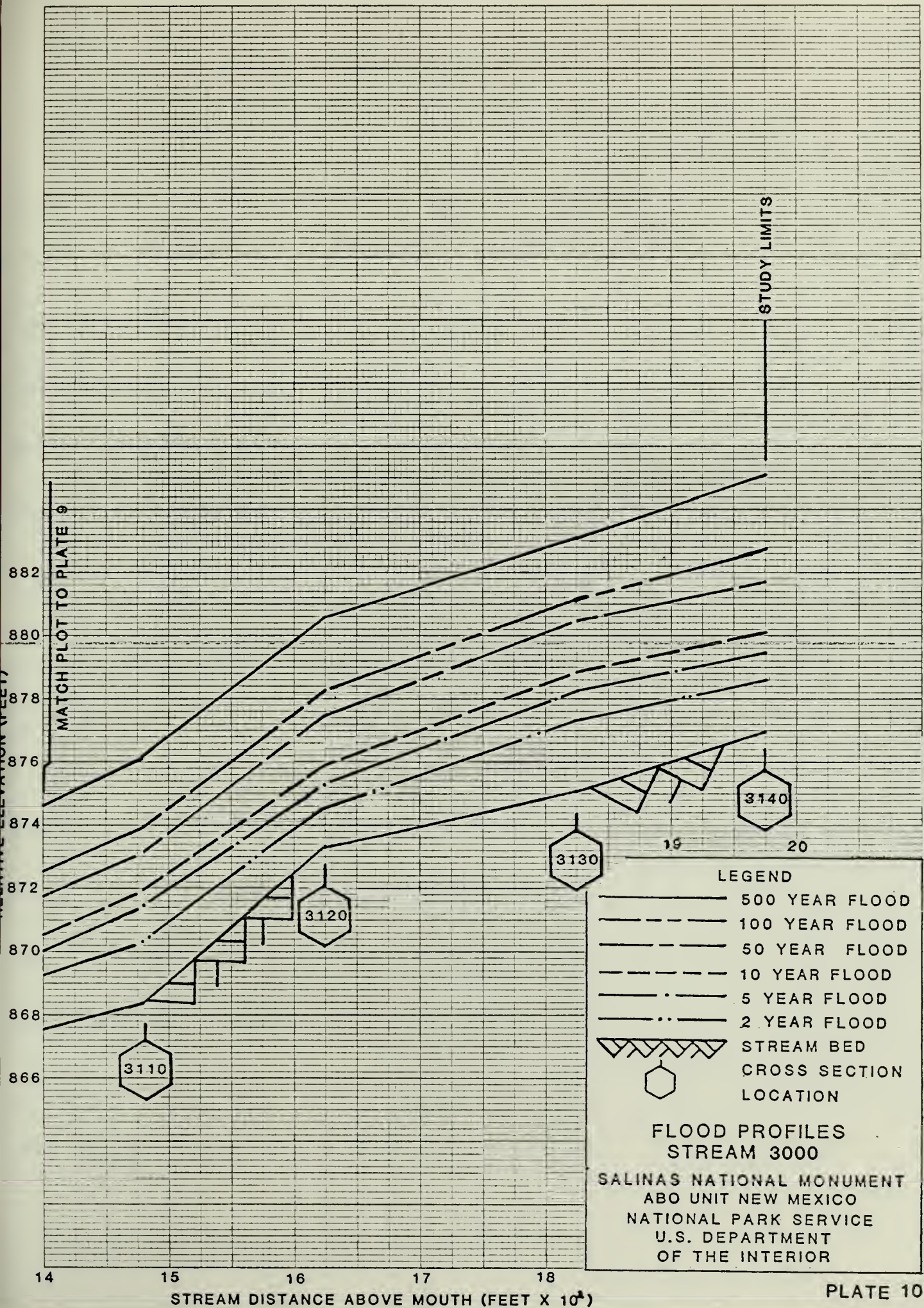


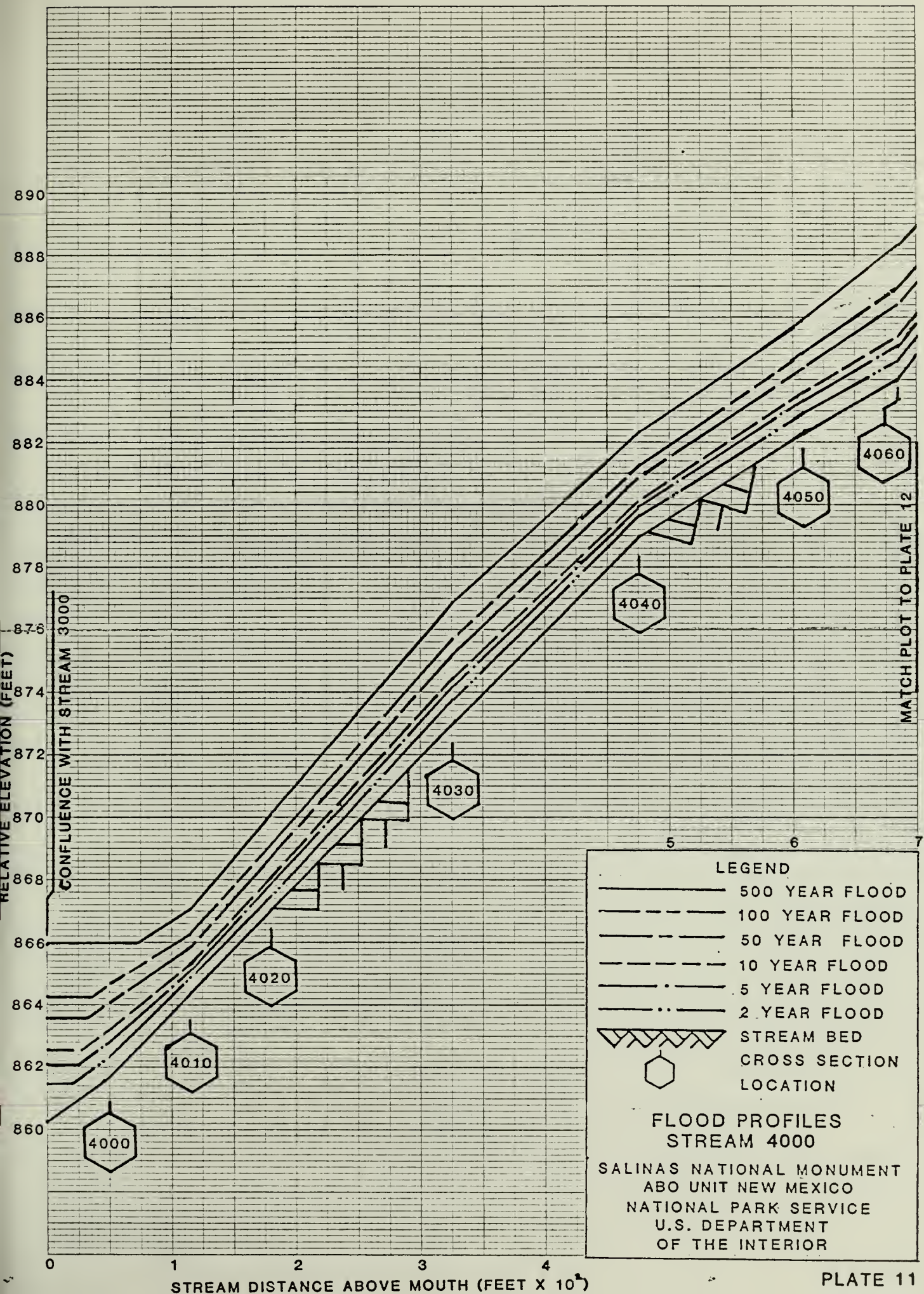


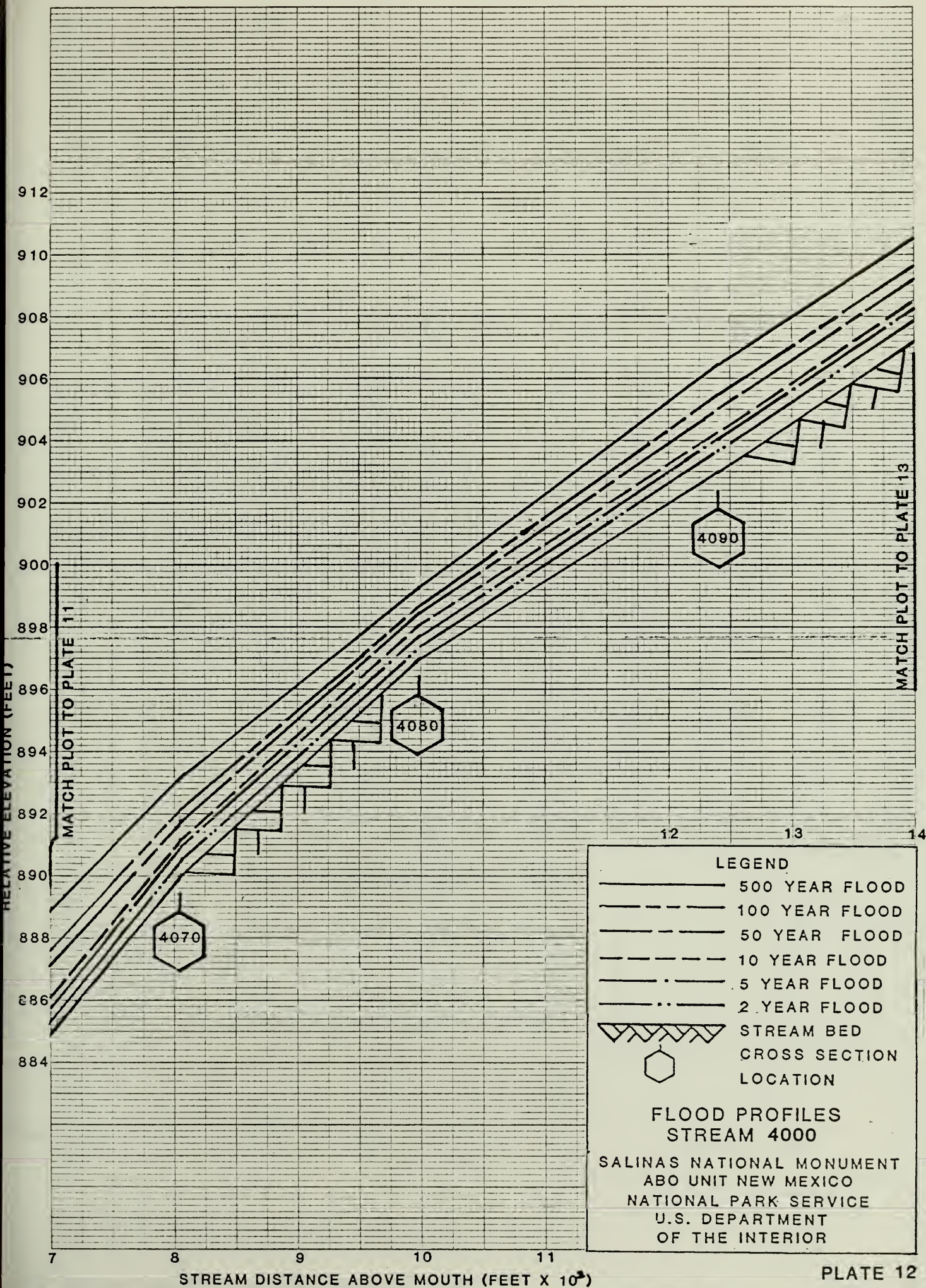




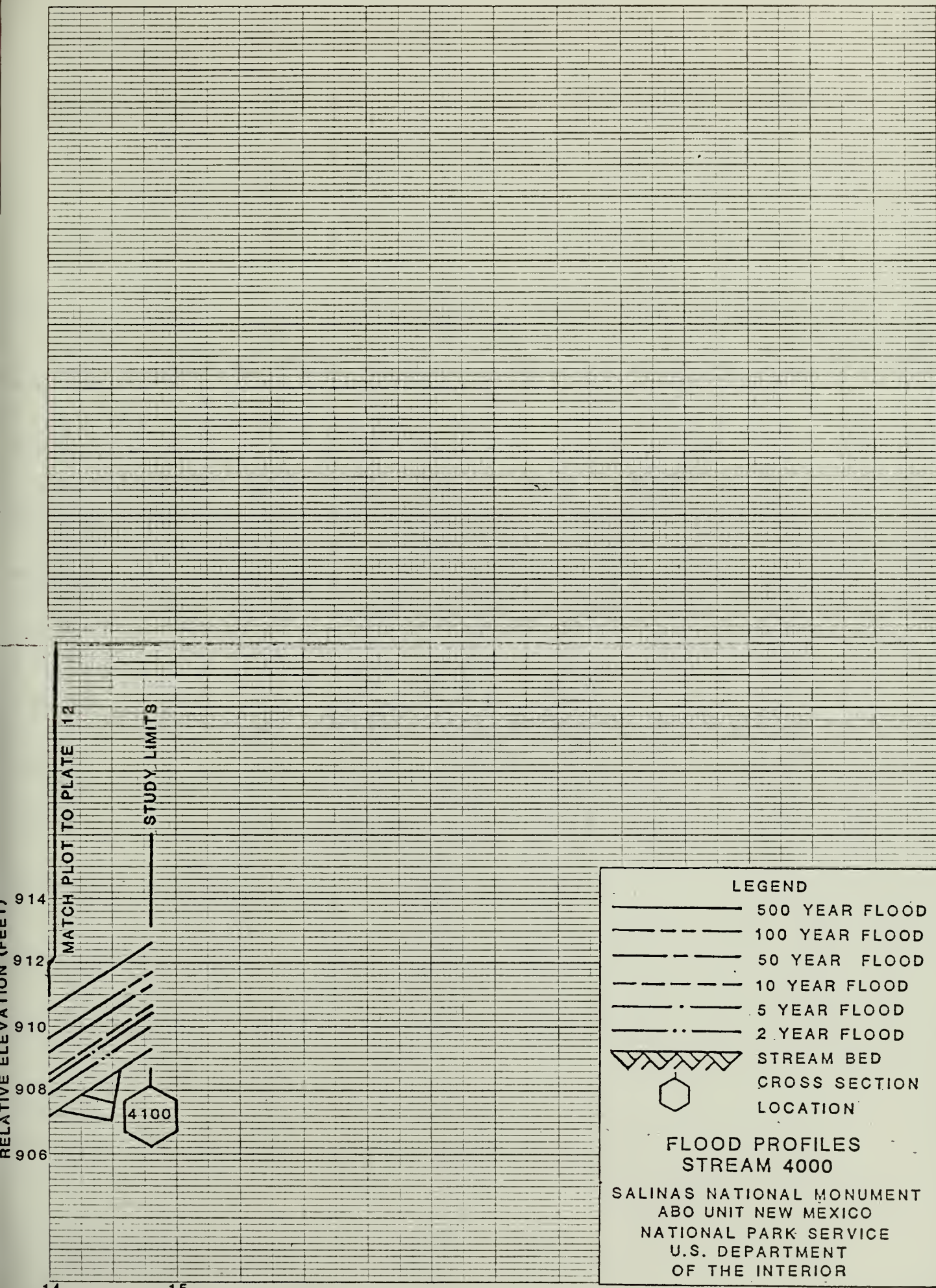












RELATIVE ELEVATION (FEET)

MATCH PLOT TO PLATE 12

STUDY LIMITS

4100

LEGEND

- 500 YEAR FLOOD
- 100 YEAR FLOOD
- 50 YEAR FLOOD
- 10 YEAR FLOOD
- 5 YEAR FLOOD
- 2 YEAR FLOOD
- STREAM BED
- CROSS SECTION LOCATION

FLOOD PROFILES
STREAM 4000

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14 15

STREAM DISTANCE ABOVE MOUTH (FEET X 10³)

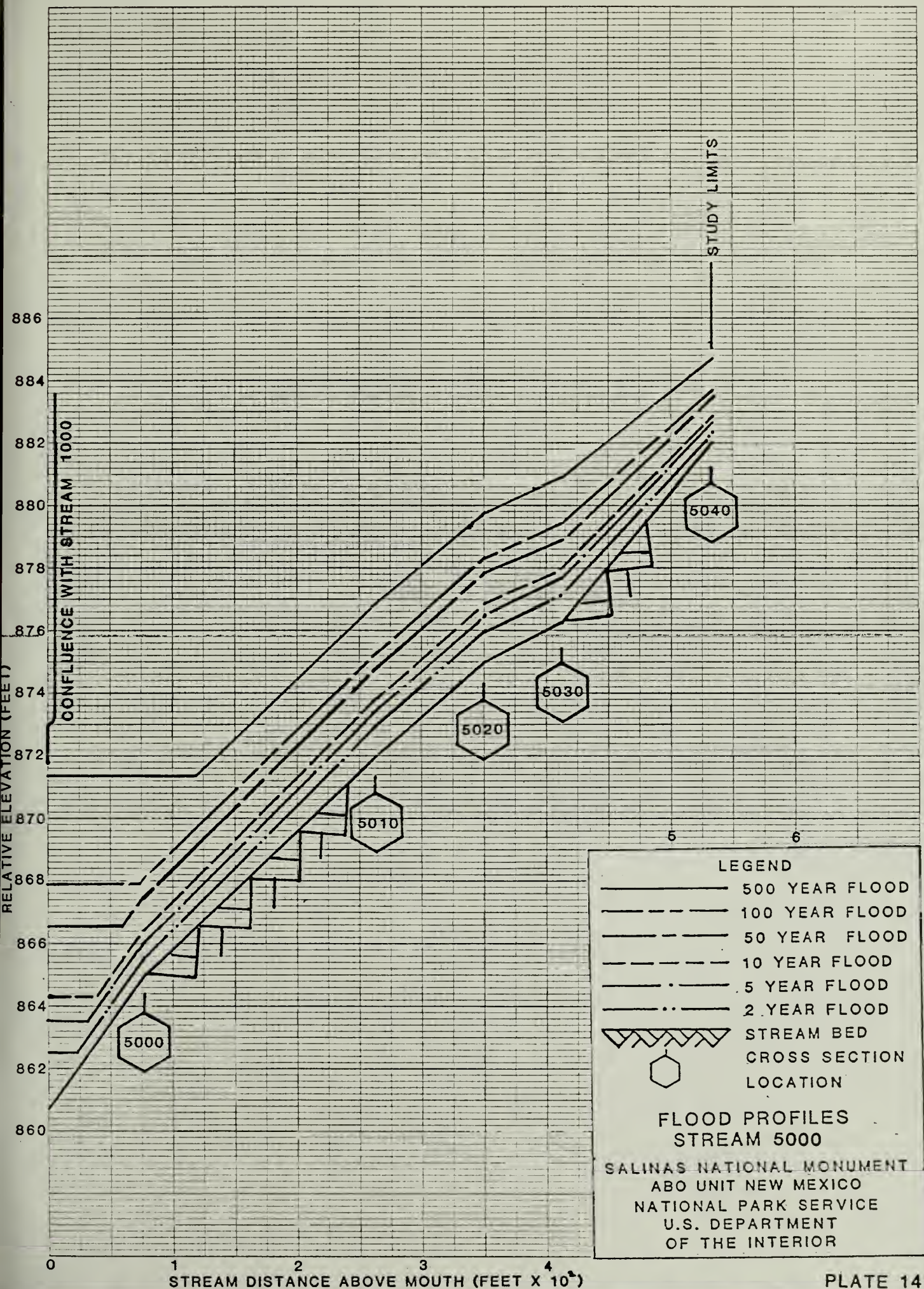


FIGURE 6

